Our Ref: 59916092:BK Contact: Bala Kilaparty

14 September 2020

Camden Council 70 Central Avenue **Oran Park NSW 2570** 

Attention: Maria Pinto

Dear Maria,

187-203 TURNER ROAD, CURRANS HILL - FLOOD IMPACT ASSESSMENT

#### 1 Introduction

A flood impact assessment (FIA) was undertaken in order to determine the potential flood impacts, if any, of the proposed development on 187-203 Turner Road, Currans Hill. The assessment was undertaken using a 1D/2D TUFLOW model that was developed to simulate overland flooding in the Narellan Creek catchment as part of Nepean River FRMSP (Cardno, 2020). The potential impact of the development was assessed under the 20% AEP, 1% AEP and PMF events. This report summarises the methodology and findings of the FIA.

The site location is shown in Figure 1-1.



Figure 1-1 Subject Site (Aerial Imagery sourced from Nearmap)



Cardno (NSW/ACT) Pty Ltd ABN 95 001 145 035

Level 9 - The Forum 203 Pacific Highway St Leonards NSW 2065 Australia

Phone+61 2 9496 7700Fax+61 2 9439 5170

www.cardno.com





### 2 Objectives

The objective of the study was to assess the flood impact of the proposed development, if any, within and near the subject site and to assess the compliance or otherwise of the proposed development works with Council's Policy.

The key tasks included:

- Running the Narellan Creek Floodplain model to estimate flooding under pre and post development conditions in the 20% AEP, 1% AEP and PMF events;
- > Assessing any impacts on flood levels, velocities and/or hazards upstream and downstream of the development; and
- > Assessing if 1% AEP floodwaters are contained within the proposed channel works within the site.

#### 3 Available Data

Cardno is preparing the Nepean River Floodplain Risk Management Study and Plan (FRMS&P) for Camden Council. The study utilises TUFLOW 1D/2D floodplain models to define flood behaviour in the Upper Nepean River and Narellan Creek located within the Camden LGA. The Narellan Creek model domain includes the subject site (187-203 Turner Road, Currans Hills).

The following data was used for this assessment:

- > The model assembled for the Narellan Creek Overland Flood Study (Cardno, 2020)
- > Survey of Existing Conditions for the subject site Provided by JMD Development Consultants on 14/08/2020
- The proposed design layout and surface levels Provided also by JMD Development Consultants on 14/08/2020

#### 4 **Pre-Development Conditions**

The pre-development scenario which was adopted was the benchmark overland hydraulic modelling performed as part of the Nepean River Floodplain Risk Management Study and Plan (Cardno, 2020). A combined 1D/2D TUFLOW model was used with the latest site survey data incorporated across the site.

The floodwaters drain from an upstream catchment along a drainage line through the site with shallow depths ranging between 0.15 - 0.7m in the 1% AEP event. High hazard zones are generally confined to the drainage line and the extent of flooding is minor outside the channel banks.

As floodwaters approach the downstream end of the site, these floodwaters overtop Turner Road. A secondary flow path is also observed in the South East corner of the site, however its extent within the site is minimal.

The hydraulic model was run for the 20% AEP, 1% AEP and PMF events. Flood results for pre-development conditions are mapped in **Figures 1 to 15** in **Appendix B.** 

#### 5 **Post-Development Conditions**

The proposed development works include constructing a channel through the site, creating 69 lots and constructing a detention basin within the site. The proposed development layout is shown in **Figure 5-1**. The proposed site layout and grading plan provided by JMD Development Consultants is attached in **Appendix A**.

59916092:BK 14 September 2020

# Cardno<sup>®</sup>



Figure 5-1 Concept Layout Plan (Source: JMD Development Consultants)

The design consists of earthworks, access roads and a proposed channel through the site. Design inputs were incorporated into the hydraulic model and the 20% AEP, 1% AEP and PMF storms were run to estimate flow behaviour under the post-development conditions. At one location, the floodwaters pass through twin culverts under a proposed road. The Manning roughness values were adjusted in accordance with the benchmark model to simulate the post-development conditions.

The flood maps generally show that floodwaters are further confined to the drainage line corridor with minor reduction to the flood extent. Flood depths up to 2m are observed towards the downstream end of the proposed channel in the 1% AEP event. Flood hazard and velocity changes are negligible within the site relative to the pre-development conditions. Floodwaters still overtop Turner Road however the hazard on the road is reduced.

#### 6 Flood Impact Assessment

The water level difference plots (see **Figures 21, 27** and **33 in Appendix B**) map the changes in flood levels due to the proposed works. No changes in flood behaviour are observed on the northern or western areas outside of the site. Due to the earthworks and proposed channel works, conveyance within the site has general improved. Floodwaters drain more efficiently and a reduction in levels is seen in the channel and on Turner Road. Reductions up to 0.06 m are observed on Turner Road in the 1% AEP event and continue further downstream along the creek reserve.

3



Minor increases of up to 0.02 m are observed towards the eastern boundary of the site on Newmarket Street in the 1% AEP event. The minor increases are confined to the road only and do not impact any properties.

Additionally flood depths on the street are less than 150mm (which is Council's adopted depth filter criteria) and it is expected that the overland flows will be collected by local drainage or confined by the kerb. It is expected these minor increases could be mitigated by the stormwater design during the detailed design stage.

Similar increases are also observed on the south eastern corner of the site. Results have been mapped in **Appendix B.** 

#### 7 Conclusions

Flood modelling has been undertaken to estimate the flood behaviour on Turner Road, Currans Hill. under the Pre and Post-development conditions.

It is concluded that:

- There is an overall decrease in flood levels as a result of the proposed works along the drainage line.
- The channel is able to contain the 1% AEP floodwaters within its banks.
- While minor increases in flood levels of up to 0.02 m were estimated on Newmarket St, largely within the road corridor, there are no measurable increases in hazard and no adverse impacts on any properties.
- On balance these impacts are therefore considered to be minor
- It is expected these minor increases could be mitigated by the stormwater design during the detailed design stage.

Yours sincerely,

Bala Kilaparty Experienced Engineer for Cardno (NSW/ACT) Pty Ltd Phone: +61 2 9496 7765

Email: bala.kilaparty@cardno.com.au

Enc: Appendix A - Proposed Design Appendix B - Flood Model Results

# APPENDIX



# **PROPOSED DESIGN**





S:\JOBS\TURNER RD\14194 TURNER RD DEV\EMW\ENG\CAD\E16 - ULTIMATE RAINGARDEN\14194E16-RAINGARDENS



# APPENDIX

# FLOOD MODEL RESULTS



59916092:BK
14 September 2020



Figure	Scenario	Title
1	Pre-Development	20% AEP Depth
2	Pre-Development	20% AEP Water Level
3	Pre-Development	20% AEP Velocity
4	Pre-Development	20% AEP General Hazard
5	Pre-Development	20% AEP Provisional Hazard
6	Pre-Development	1% AEP Depth
7	Pre-Development	1% AEP Water Level
8	Pre-Development	1% AEP Velocity
9	Pre-Development	1% AEP General Hazard
10	Pre-Development	1% AEP Provisional Hazard
11	Pre-Development	PMF Depth
12	Pre-Development	PMF Water Level
13	Pre-Development	PMF Velocity
14	Pre-Development	PMF General Hazard
15	Pre-Development	PMF Provisional Hazard
16	Post-Development	20% AEP Depth
17	Post-Development	20% AEP Water Level
18	Post-Development	20% AEP Velocity
19	Post-Development	20% AEP General Hazard
20	Post-Development	20% AEP Provisional Hazard
21	Post Less Pre-Development	20% AEP Water Level Difference
22	Post-Development	1% AEP Depth
23	Post-Development	1% AEP Water Level
24	Post-Development	1% AEP Velocity
25	Post-Development	1% AEP General Hazard
26	Post-Development	1% AEP Provisional Hazard
27	Post Less Pre-Development	1% AEP Water Level Difference
28	Post-Development	PMF Depth
29	Post-Development	PMF Water Level
30	Post-Development	PMF Velocity
31	Post-Development	PMF General Hazard
32	Post-Development	PMF Provisional Hazard
33	Post Less Pre-Development	PMF Water Level Difference

7

































































